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Should the Drift Chamber Inner Cylinder Be Load Bearing?

Abstract

Chamber endplates in which the inner and outer radii are at the same z coordinate (such as both endplates of the BaBar drift chamber) suffer large deflections under the wire load if they are only supported at the outer radius. By making the inner cylinder load bearing the deflections of the endplates would be reduced by a factor of ≈ 30 . This would permit the use of thinner endplates, or simplify the prestressing operation, or both. Of course, both the inner and outer cylinders must be stable against buckling under the wire load. Calculations indicate that a $540-\mu$ m-thick carbon-fiber inner cylinder would be just at the buckling limit, while a 1.5-mm-thick cylinder would buckle only at 10 times nominal load. A recent destructive test at Princeton supports these calculations. A 1.5-mm-thick inner cylinder would present 0.7% radiation lengths to particles, a modest increment to that of the adjacent PEP-II support tube (presently spec'ed to be 1.2% of a radiation length, not counting aluminum needed for an rf shield).

1 Deflection of Flat Annular Plates Under Uniform Loading

There exist analytic solutions to the deflections and stresses of flat circular endplates under uniform loads, summarized in the Roark's Formulas for Stress and Strain by W.C. Young (6th ed., McGraw Hill, 1989). These formulae are included as part of TK Solver, a PC-based engineering spreadsheet program (Universal Technical Systems, Rockford, IL, 815-963-2220).

We have used the formulae of Chapter 10, Table 24, Case 2 to calculate the measured deflections of several existing drift chambers, whose properties are surveyed in Table 1.

Among chambers with flat endplates, we calculated the deflections of the CLEO II, CDF and ZEUS chambers with results summarized on pages 8-10. The agreement is rather good using values of Young's modulus for aluminum reduced by the fraction of the endplate area lost to holes. The CLEO II endplate is supported only at its outer edge, while the CDF and ZEUS endplates are supported at both inner and outer radii.

Calculations for the proposed 32-mm-thick rear endplate of the BaBar drift chamber are presented on p. 11, with results very similar to those reported earlier by C. Hearty (who used the same formula from Roark and Young); namely a maximum deflection of 3.3 mm under a load of 3500 kg. The plate deforms into a very shallow cone of angle 5 mrad.

Then on p. 12 we show results assuming both the inner and outer edges of the plate are supported; in this case the maximum deflection is only 100 μ m, and the 3500 kg load is distributed with 2100 kg at the outer edge and 1400 kg at the inner. A cross section of the plate is now a shallow parabola, and the angles at the edges are less than 1 mrad.

Thus the calculations indicate a factor of 28 reduction in the deflection of the flat endplate when both edges are supported compared to supporting only the outer edge.

A preliminary finite element analysis of the proposed biconical carbon-fiber endplate using the PC-based program ALGOR indicates a factor of 8 reduction in the maximum deflection when both edges are supported compared to supporting only the outer edge. Details of this analysis will be presented in a separate note.

The deflection of a flat plate is almost entirely due to bending (as contrasted with elongation), and so varies as $1/t^3$ where t is the thickness of the plate. For example, a 16-mm-thick plate would deflect by 0.8 mm.

The additional results on pp. 13-14 hold if the edge (or edges) of the plate are clamped

rather than simply supported at previously assumed. Clamping (= constraining the slope of the plate as well as the position) would reduce the deflection by an additional factor of 5.

However, the clamping condition is unrealistic if the endplate is simply attached to a thin-wall support tube; in practice the end of the support tube would provide very little resistance to the bending of 1-5 mrad at the edges of the endplate. A massive stiffening ring would be require to provide the clamping condition.

2 Buckling of a Cylindrical Shell Under Axial Load

If the proposed endplates were supported at both inner and outer edges the deflection under the wire load would be only 100-200 μ m. This is so small that no prestressing of the endplates would be needed. This advantage would be best realized if both outer and inner support tubes were mounted together with the endplates before wire stringing, and the stringing performed with the chamber axis vertical (as for the CLEO II and ZEUS chambers).

A critical issue is the thickness of the inner support tube so that it is stable against buckling.

A well-known result for long columns is the Euler condition for the maximum axial force before the column deforms into an arc (picturesquely demonstrated by Charlie Chaplin's cane):

$$
F_{\text{max}} = \frac{4\pi^2 EI}{l^2},\tag{Euler}
$$

where E is the modulus of elasticity (Young's modulus), I is the area moment of inertia, and l is the length. For a tube of radius r and thickness t the momentum of inertia is $I = \pi r^3 t$, so we have

$$
F_{\text{max}} = \frac{4\pi^3 E r^3 t}{l^2}, \qquad \text{(tube)}.
$$

For solid columns Euler calculated that buckling into higher-order modes required a larger axial force than that for the fundamental mode just presented. Around 1910, Lorenz, and also Timoshenko, realized that for thin-walled tubes it is easier to buckle into a higher-order mode. They first considered modes corresponding to longitudinal waves and later considered combined longitudinal and azimuthal waves to find in both cases that

$$
F_{\text{max}} = \frac{2\pi E t^2}{\sqrt{3(1 - \nu^2)}},
$$
 (Timoshenko),

where ν is Poisson's ratio (the ratio of transverse to longitudinal strain of a material under longitudinal stress). This result is remarkable in that neither the length nor the radius of the tube appears. The characteristic wavelength of the failure mode is small compared to the both the length and radius of the tube, in contrast to Euler's result for solid columns.

The failure mode for combined longitudinal and azimuthal modes is sometimes called the Yoshimura pattern, illustrated in the figure on p. 4 (taken from Buckling of Shells for Engineers by L. Kollár and E. Dulácska, Wiley, 1984). In this pattern the surface of the tube breaks up into flat triangular facets. Experiment shows that the Yoshimura pattern is the typical failure mode for thin tubes.

Fig. 2.6. The Yoshimura-pattern

However, experiment also shows that the force at which buckling occurs is typically less than that given by Timoshenko's result, especially for long thin tubes. Some data are summarized in the figure at the top of p. 5 (taken from Buckling of Bars, Plates, and Shells by D.O. Brush and B.O. Almroth, McGraw-Hill, 1975). Here $k_a \equiv F_{\text{max}}l^2(1-\nu^2)/(2\pi r t^3 E)$, $a \equiv r$ and $h \equiv t$. A number of explanations for this discrepancy have been put forward, but it is very suggestive that the experimental data for buckling under axial compression appear to follow the functional form of theory (and experiment) for buckling under torsion. See the lower figure on p. 5. The buckling may be provoked by a slight torsion due to asymmetric loading or imperfection in the tube. Therefore I adopt the semi-empirical functional form

$$
F_{\text{max}} = \frac{\pi^3 E t^{9/4} r^{1/4}}{6 l^{1/2} (1 - \nu^2)^{5/8}},
$$
 (semi-empirical),

which is a translation of the fit $k_a = Z^{3/4}$ to the data on both axial and torsional buckling.

2.1 Laboratory Test

The buckling force predicted by the above formula is remarkably large. We felt the need to perform a lab test to check it.

We rolled a cylinder of radius $6''$ and length $36''$ from a sheet of G-10 about $0.018'' =$ 450 μ m thick. The seam of the tube consisted of a 1" overlap secured with pop rivets every

inch. A data sheet lists the modulus of G-10 as 2.5×10^6 psi. We assume the Poisson ratio is 0.3. The calculated buckling force is then 425 pounds.

We placed a plate on the top end of the tube and carefully added lead bricks until the tube buckled – at a load of 715 pounds! The failure pattern was of the Yoshimura type, but restricted to the upper and lower 1/5 of the tube. See the photo below.

We thus obtain some confidence that the semi-empirical formula represents a lower bound on the buckling force for tubes similar to that proposed for BaBar.

2.2 Proposal for the BaBar Drift Chamber

The support tubes for the BaBar chamber are to be made of filament-wound carbon-fiber epoxy. Tubes fabricated from this material can be obtained with a Young's modulus at least 1.4×10^{11} Pascals. We take the Poisson ratio to be 0.3.

According to the calculations in sec. 1, the 3500-kg wire load should be distributed with 2100 kg on the outer tube and 1400 kg on the inner tube for minimum deflection of the rear endplate.

Then according to the semi-empirical formula, the thickness of 3-mm-long tubes just at the buckling limit is 540 μ m for the inner tube, and 570 μ m for the outer tube. For safety we design the buckling load to be 10 times the nominal load, requiring tubes 2.8 times thicker: 1.5 mm thick for the inner tube and 1.6 mm thick for the outer tube.

3 References

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